UNIVERSITY^{OF} BIRMINGHAM University of Birmingham Research at Birmingham

Prediction of thermal-induced buckling failures of ballasted railway tracks using Artificial Neural Network (ANN)

Ngamkhanong, Chayut; Kaewunruen, Sakdirat

DOI: 10.1142/S0219455422500493

License: None: All rights reserved

Document Version Peer reviewed version

Citation for published version (Harvard):

Ngamkhanong, C & Kaewunruen, S 2022, 'Prediction of thermal-induced buckling failures of ballasted railway tracks using Artificial Neural Network (ANN)', *International Journal of Structural Stability and Dynamics*, vol. 22, no. 5, 2250049. https://doi.org/10.1142/S0219455422500493

Link to publication on Research at Birmingham portal

Publisher Rights Statement:

Electronic version of an article published as International Journal of Structural Stability and Dynamics, Volume, Issue, Year, Pages 10.1142/S0219455422500493 © copyright World Scientific Publishing Company https://doi.org/10.1142/S0219455422500493

General rights

Unless a licence is specified above, all rights (including copyright and moral rights) in this document are retained by the authors and/or the copyright holders. The express permission of the copyright holder must be obtained for any use of this material other than for purposes permitted by law.

•Users may freely distribute the URL that is used to identify this publication.

•Users may download and/or print one copy of the publication from the University of Birmingham research portal for the purpose of private study or non-commercial research.

•User may use extracts from the document in line with the concept of 'fair dealing' under the Copyright, Designs and Patents Act 1988 (?) •Users may not further distribute the material nor use it for the purposes of commercial gain.

Where a licence is displayed above, please note the terms and conditions of the licence govern your use of this document.

When citing, please reference the published version.

Take down policy

While the University of Birmingham exercises care and attention in making items available there are rare occasions when an item has been uploaded in error or has been deemed to be commercially or otherwise sensitive.

If you believe that this is the case for this document, please contact UBIRA@lists.bham.ac.uk providing details and we will remove access to the work immediately and investigate.

- 1 Prediction of thermal-induced buckling failures of ballasted railway tracks using Artificial
- 2 Neural Network (ANN)
- 3 Chayut Ngamkhanong¹, Sakdirat Kaewunruen²
- ⁴ ¹ Department of Civil Engineering, Faculty of Engineering, Chulalongkorn University, 254 Phayathai
- 5 Road, Pathumwan, Bangkok, 10330, Thailand; <u>chayut.ngam@gmail.com</u>
- ⁶ ² Department of Civil Engineering, School of Engineering, University of Birmingham, Edgbaston B15
- 7 2TT, UK; <u>s.kaewunruen@bham.ac.uk</u>

8 Abstract

9 The present study investigates the potential of the implementation of machine learning techniques in analysing the buckling phenomena of ballasted railway tracks induced by extreme temperature. In this 10 study, Artificial Neural Networks (ANNs) have been established to determine the design relationship 11 12 between various ballasted track conditions and outputs namely safe temperature and buckling temperature. 13 The variables that are taken into account in the objective function of the optimisation problem are the lateral 14 resistance of ballasted track provided by ballast-sleeper, torsional resistance provided by fastening systems, and dimensioning of the cross sections. Due to its complexity in parameter combinations, the objective of 15 the study is to create predictive models with the aim of minimising the usage of scarce resources. 16 17 Comprehensively, all three hundred and fifty-three datasets of the safe and buckling temperatures derived 18 from previous FE results have been collected and trained. The optimal ANN architecture with a very high 19 rate of accuracy has been determined. Note that the performance of the optimal ANN architecture has been 20 assessed by employing the mean squared error (MSE) and the coefficient of determination (R^2). Thus, the 21 neural network model can be applied to help estimate buckling temperature for the complex track models 22 in order to detect track buckling in summer.

Keywords: ballasted railway track, track buckling, extreme temperature, structural stability, artificial
 neural network, machine learning.

25 **1 Introduction**

At present, railway track buckling is one of the serious issues in the railway system [1-3]. Hence, railway infrastructure developments related to adaptation to future heatwave are expected. This is because the

- summer heat can significantly increase the rail temperature and cause the rail to expand, leading to a build-
- 29 up of axial compression force in continuous welded rail (CWR). Although CWR provides a smooth ride
- 30 and has a lower maintenance cost in comparison to a jointed rail, it still suffers from the drawbacks as the
- 31 track can lose its lateral alignment and get buckled when the rail temperature reaches a certain limit [4-7].
- Based on the evidence [8-10], track buckling can cause derailment and cause a huge loss of assets and can
- also result in the loss of passenger lives. It is important to note that track buckling around the world usually
- 34 occurs in conventional railway ballasted tracks due to the poor track conditions and track lateral
- 35 misalignment. Buckling analysis has been widely performed to obtain the significant parameters and track
- 36 conditions influencing track buckling and corresponding safe and buckling temperatures [11-14].
- 37 Previous studies showed that lateral resistance provided by ballast and sleeper is the major factor
- 38 influencing the buckling strength and buckling phenomena [15]. The lateral resistance can be evaluated by
- 39 performing a Single Sleeper (Tie) Push Test (STPT) [16-19]. This method provides the ballast-sleeper
- 40 contact force encountering sleeper movement which can be represented as a track lateral resistance. While

41 the minor factors include torsional resistance between sleepers and rails provided by fasteners. It is

- 42 important to note that different factors can significantly create different phenomena of railway tracks
- 43 exposed to extreme temperature. It is noted that the traditional beam theory is very limited in proposing a
- 44 number of parameters into the calculation of the buckling temperature [20, 21]. Hence, FE model can help
- 45 achieve more complex structures with more parameters. However, FE analysis uses high computational
- 46 time and needs a large memory consumption. As a result, to further expand the previous results from FE 47 model, machine learning can be used to save time and increase efficiency in predicting buckling phenomena
- 47 model, machine learning can be used to save time and increase efficiency in predicting bucking phenomena 48 of railway tracks. It is important to note that these parameters are compounded and create realistic results
- 49 from the complicated valuables that cannot be predicted analytically.
- 50 An artificial neural network (ANN), a type of machine learning, is a simplified mathematical model that
- 51 can simulate the function of natural biological neural networks to learn from the experience for solving new 52 problems. Unlike traditional model-driven methods, machine learning is a novel data-driven method to
- 53 explore and build algorithms that learn from known data and predict unknown data. Most traditional
- statistical models have certain requirements or assumptions for the specific distribution of data. However,
- 55 in reality, data may not meet those assumptions or requirements. In this case, the machine learning method
- is more reasonable. The machine learning method does not need to make any assumptions about the data
- distribution, and the results produced can also be evaluated by a cross-validation method [22]. Machine
- 58 learning technique has been widely used in civil and railway applications especially for predicting the
- 59 properties of materials of track components based on the proportion of the ingredients e.g. recycled concrete
- 60 [23], rubberised concrete [24] etc. More examples on railway track applications includes track response
- 61 quantification [25], train weight prediction [26], fault detection [27-30], railway safety and accident 62 identification [31, 32]. Hence, this paper is the first to use machine learning in predicting buckling
- 63 phenomena of ballasted railway tracks.
- 64 The aim of this paper is to establish a machine learning approach capable of predicting the safe and buckling 65 temperatures considering various compositions of parameters. This paper presents the optimal architecture
- 66 for estimating the safe and buckling temperatures of ballasted railway tracks. The datasets have been
- 67 collected from the previous numerical studies based and are input into different ANN models to quantify
- the performances and obtain the best model [33, 34]. The optimal ANN models are found to be capable of
- 69 managing the complicated relationships between the inputs and outputs and of designing track components
- to encounter the extreme temperature. The ANN models are then compared with the traditional multiple
- 71 linear regression models and show a higher accuracy in comparison to the multiple linear regression models.
- 72 The suggested ANN architecture can be used further in order to accurately predict the buckling phenomena
- and help improve track conditions to encounter extreme heat in summer.

74 2 Railway Track Buckling

- 75 According to previous studies, the major parameters affecting buckling strength of ballasted railway tracks is the lateral resistance provided by ballast and sleeper components. The torsional resistance by fastening 76 77 systems is also considered. In general, track lateral misalignment can be a significant factor that undermines 78 the buckling strength. This study also includes the effect of unconstrained length that can help increase the 79 lateral restraint. This method can be either a spot replacement method or improving ballast-sleeper constrain 80 at particular spans. Note that only a few studies have considered an unconstrained length in buckling 81 analysis. Figure 1 shows the arrangement of typical ballasted track with the idealised tensionless spring 82 representing the lateral stiffness of ballasted track. It also presents the lateral resistance curve that can be represented by the tensionless lateral spring connected to the sleeper ends. Note that the lateral resistance 83
- 84 curve is usually derived by the sleeper push test. The elastoplastic curve generally presents the resistance

- 85 curve considering the lateral resistance force (F_p) and the sleeper displacement. Note that the lateral stiffness
- 86 is the lateral resistance force over the displacement limit of sleeper (F_p/W_p) .



Figure 1 Ballasted railway track.

89 In buckling mechanism, if the rail temperature is over the neutral temperature or stress-free temperature, 90 the compression axial force in the rails builds up. The rail can be buckled when the compression force 91 reaches its limit or buckling resistance. It should be noted that buckling resistance is affected by track and 92 element types and track conditions. The relationship between rail temperature and lateral displacement is 93 typically plotted as seen in Figure 2. It can be seen that there are two types of buckling depending on the 94 post-buckling path: sudden buckling and progressive buckling. In the pre-buckling stage, the rails are 95 exposed to the temperature over neutral temperature and the axial force is linearly increased. As for the 96 sudden buckling (also called "Snap-through"), the track buckles explosively with no external energy after 97 reaching its maximum temperature (upper critical temperature, T_{Bmax}) and becomes unstable in its post-98 buckling stages. This temperature can be defined as a "buckling temperature". Besides, T_{Bmin} represents the 99 lower bound which can buckle the track if sufficient energy is supplied. It can also be defined as a "safe 100 temperature" since the track cannot buckle if it experiences a temperature below this temperature. 101 Moreover, progressive buckling can occur when the T_{Bmin} cannot be differentiated from T_{Bmax} . In this case, track lateral displacement is gradually increased after buckling and the critical temperature is defined as T_P. 102 103 In this study, both minimum and maximum temperatures will be predicted using ANN. Note that the snap-104 through buckling mode can be obtained when the minimum and maximum temperatures are obtained while 105 the progressive buckling failure modes can be obtained when temperatures are crossed over meaning that 106 the only one output is predicted.

107

88

108

Figure 2 Buckling path.

109 **3 Methodology**

An artificial neural network (ANN) is a data prediction framework based on existing features created from the human mind structure. It simulates the processing mechanism of the human brain's nervous system to complex information. A neural network is a computational model consisting of a large number of nodes (or neurons) connected to each other. This network is made of some functional blocks, which are named neurons. Neurons are connected by weights, which are usually randomly selected at first. Weights in a

- 115 learning process are increased or decreased by some epochs to eventually achieve the desired network
- 116 which can predict it by reasonable accuracy [22].

117 Therefore, in a trained neural network, the desired output can be achieved by receiving the inputs

118 considering the updated weights as shown in Figure 3. The network improves over time by comparing the 119 desired input and output calculating the error. The improvement of the machine learning model from time

120 to time indicates the accuracy if the prediction model can be improved and the predicted results are reliable.

- 121 Usually, nonlinear activation functions like sigmoid (tansig and logsig) are used because of its better
- response than others.
- 123

124

Figure 3 Artificial neural model.

The general ANN architecture includes three main layers: input layer, hidden layer and output layer, as shown in Figure 4. This figure shows the multilayer feed forward network that is considered in this study. The input layer receives various forms of information from the outside. This is the information that the neural network is designed to process or learn. Data from the input layer passes through the hidden layers that consist of the number of hidden neurons. Role of the hidden layers is to convert the input information

- 130 into content that the output layer can use.
- 131

132

Figure 4 ANN architecture.

133 **3.1 Data collection**

134 The datasets have been collected from the previous numerical studies [33, 34]. Table 1 presents the input 135 parameters considered in this study. The input includes major parameters affecting track buckling strength: lateral stiffness, displacement limit of sleeper, fastening torsional system, track lateral misalignment. Track 136 137 unconstrained length is also considered to help strengthen the buckling strength by restraining the track laterally to encounter the lateral movement of sleeper that can be achieved by spot replacement method [14, 138 139 34, 35]. Due to the limited studies on the influences of track unconstrained length, 350 datasets are 140 introduced to the ANN models for predicting the output parameters. Note that the output variables are the 141 safe and buckling temperatures. These two values can also help predict the buckling failure modes. Snap-142 through buckling mode occurs If the buckling temperature is higher than safe temperature. However, when 143 the two parameters are crossed over (safe temperature > buckling temperature) meaning that only one output 144 is presented while another is a trivial solution. This represents the progressive buckling mode since only 145 one temperature is obtained.

146

Table 1 Input parameters.

Input parameters	Range	Unit
Lateral stiffness, $F_p/W_p(x_1)$	200-2600	N/mm
Displacement limit of sleeper, $W_p(x_2)$	0.5-2	mm
Fastening torsional resistance (x_3)	75-225	kNm/rad
Track lateral misalignment (x_4)	8-32	mm
Track unconstrained length (x_5)	6-30	m

147

148 **3.2 ANN architecture**

149 All imported datasets are randomly divided into three parts for training, validation and testing, respectively.

150 The fixed allocation ratios of datasets between training, validation and testing aspects are 70%, 15% and

151 15%, respectively. The datasets for training are utilised for training models by modifying weights. The 152 validation sets are used to adapt the model selection, that is, to do the final optimisation and determination 153 of models, such as choosing the number of hidden neurons and hidden layers, while the testing set is purely 154 to prove the generalisation of the trained models. In this study, the number of neurons is varied from 1 to 155 15. The model description is written in the form of "input parameters – hidden layer – output parameters". 156 Table 2 presents ANN models with different explications (hidden neurons)

156 Table 2 presents ANN models with different architectures (hidden neurons).

157

Table 2 ANN architecture models used in this study.

Model	ANN Architecture
1	512
2	552
3	5 10 2
4	5 15 2

158

This paper uses Levenberg–Marquardt (LM) as an algorithm to predict the output parameters. LM is an algorithm that provides a solution of the numerical nonlinear minimisation. The significance of LM algorithm is that it can simultaneously achieve the advantages of the Gauss–Newton method and the gradient descent algorithm by changing parameters. Furthermore, LM algorithm can improve the shortcomings of other algorithms. The LM algorithm is a type of upgraded Newton method shown in Eq. (1)

$$x_{k+1} = x_k - [J^T J + uI]^{-1} J^T e$$
(1)

where, I indicates the identity matrix, e represents a vector, J is a Jacobian matrix, x_k denotes the weight at epoch K, and u is a damping factor. In order to improve the accuracy, u can be increased or dropped

according to the success or failure of steps, and then the performance function can be enhanced.

169 **3.3 Performance Evaluation of ANN Architecture**

170 In order to investigate the performance of the trained models in this study, two statistical analyses named 171 Mean Squared Error (MSE) and the coefficient of determination (R^2 value) are employed. MSE is the 172 average value of the cost function for minimising the sum of squared errors (SSE) during the linear 173 regression model fitting process. This represents the mean square error between the predicted and the actual 174 value. MSE value is calculated according to Eq. (2) [36]. The lower MSE value indicates a model with 175 higher accuracy.

$$MSE = \frac{1}{n} \sum_{i=1}^{n} (y'_{i} - y_{i})^{2}$$
⁽²⁾

177 where n is the number of samples, and $(y'_i - y_i)$ is the result of the input value minus the predicted value on the testing sets being processed. Moreover, R^2 value is employed as an assistance method to determine 178 the performance of trained models defined in Eq. (3). R^2 value exhibits the percentage of real value changes, 179 180 which can be influenced by the variation of the predicted value. The range of R^2 value is from zero to one. 181 Considering Eq. (3), the numerator part represents the sum of the squared difference between the real value 182 and the predicted value, similar to MSE. The denominator part represents the sum of the squared difference between the real value and the mean [37]. If the result is close to 0, it means that the model fits poorly. If 183 184 the result is close to 1, it means that the model is error-free. Generally, the larger the R^2 value is, the better 185 the model fitting effect is.

$$R^{2} = 1 - \frac{\sum_{i=1}^{n} (y'_{i} - y_{i})^{2}}{\sum_{i=1}^{n} (y_{i} - \bar{y})^{2}}$$
(3)

187 **4 Results and Discussions**

186

188 **4.1 Optimal ANN architecture**

189 In this study, four models of ANN architecture are firstly considered and compared to potentially create the best ANN model for predicting the safe and buckling temperatures of ballasted railway tracks. It is found 190 191 that only one hidden layer can appropriately provide high accuracy of the prediction values compared to 192 the target values. The number of neurons is varied from 1 to 15. The model description is written in the form of "input parameters - hidden layer - output parameters". Figure 5 presents the predicted values 193 194 employed by the first ANN architecture 5-1-2 against the actual or target values. The R² values for safe and 195 buckling temperatures are presented separately. It is shown that this architecture should not be used to as a 196 predictive model since it cannot not perform well as shown by the low R² value especially for the safe 197 temperature. Furthermore, the number of neurons must be increased to improve the performance of ANN 198 model.



(a)

(b)

(c)

Figure 5 Predicted value vs actual value analysed by model 1 (5-1-2): a) safe temperature b) buckling temperature and c) Both safe and buckling temperatures.

201 After increasing the number of neurons until the performance is accepted, Figure 6 presents the predicted temperatures based on Model 4 against the actual temperatures of buckling phenomena of ballasted railway 202 tracks. This model includes 15 neurons in the hidden layer. It is clear that the R^2 values are significantly 203 204 higher than those obtain in Model 1 with lesser neurons. The R² value of safe temperature prediction model 205 is much higher than that in Model 1 and the curve shows a significant better trend in comparison to Model 1. Table 3 compares the performance of each model for both output predictions. The prediction 206 207 performances of both outputs show the similar trends. It is clear that increasing a number of neurons can significantly reduce MSE and increase R². It should be noted that Model 4 has R² value of 0.9921 which is 208 fairly high leading to the fact that increasing a number of neurons to over 15 will not improve the 209 210 performance of the model. On this ground, it can be concluded that, based on the provided datasets, 1 hidden 211 layer with 15 neurons can give a high accuracy for predicting safe and buckling temperatures of ballasted railway tracks considering different track conditions. 212

(a)

(b)

(c)

Figure 6 Predicted value vs actual value analysed by model 1 (5-15-2): a) safe temperature b) buckling temperature and c) Both safe and buckling temperatures.

215 Table 3 Mean square error and R-square values of each model architecture.

Model	Architecture	MSE			R ²		
		T_{min}	T _{max}	Total	T_{min}	T _{max}	Total
1	5-1-2	185.46	134.46	159.96	0.4091	0.8797	0.8111
2	5-5-2	26.63	53.29	39.96	0.9139	0.9501	0.9517
3	5-10-2	12.96	62.30	37.63	0.9584	0.9432	0.9554
4	5-15-2	7.75	5.40	6.57	0.9753	0.9949	0.9921

216

217 **4.2 Comparative analysis**

As shown previously, Model 4 represents the best ANN architecture for predicting the safe and buckling temperatures of railway tracks. This model is chosen to be used from now on to compare with another prediction method and perform parametric study. It is noted that linear regression is commonly used to obtain the relationship between the dependent and independent variables. In this section, linear regression with multiple variables (also called Multi Linear Regression (MLR)) is used to compare the results with the ANN model. Note that MLR is a simple and well-known technique that can help to establish a relationship between the factors and predicted output values. The general equation of MLR is presented in Eq. 4.

225 $y = a_0 + a_1 x_1 + a_2 x_2 + \ldots + a_m x_m$ (4)

where, y is the predicted value, x_m is the independent variables, a_0 is the y-intercept, and a_m indicates the regression coefficients. It is noted that MLR is conducted using data analysis tools in Microsoft Excel. The regression coefficients are obtained while x_1 , x_2 , x_3 , x_4 , and x_5 represent the lateral resistance, lateral displacement limit, torsional resistance, unconstrained length, and lateral misalignment, respectively. After obtaining the coefficients, the equations for predicting safe and buckling temperatures can be drawn separately as shown in Eqs. 5-6.

232
$$y_1 = 50.731 + 0.00878x_1 + 7.265x_2 + 0.080x_3 - 1.624x_4 - 0.032x_5$$
 (5)

233

$$y_2 = 77.458 + 0.0294x_1 + 15.609x_2 + 0.034x_3 - 1.335x_4 - 2.096x_5$$
(6)

234 Where y_1 and y_2 represent the safe temperature and buckling temperature, respectively.

The prediction accuracy of both models can be defined by the MSE and R^2 as presented in Table 4. It is evident that the R^2 values of MLR are relatively low while the MSE values are significantly high compared to those obtained by the optimal ANN model. These phenomena are observed when predicting both output values. On this ground, it can be concluded that ANN model gives a higher accuracy for predicting buckling phenomena than the traditional MLR method. This is because of the ability of MLR that can only describe the linear relationship between input and output parameters only while, in fact, the relationship between track conditions and buckling phenomena is more complex than linear relationship.

242

 Table 4 Comparison between MLR and ANN (5-15-2).

Dradiated values	М	SE	R ²		
Fledicted values	MLR	ANN (5-15-2)	MLR	ANN (5-15-2)	
T _{min}	56.57	7.75	0.8182	0.9753	
T _{max}	191.03	5.40	0.8204	0.9949	

243

244 **4.3 Parametric study**

After obtaining the optimal ANN architecture, the approximate general functions can be employed considering the weighted inputs and the transfer function to create the outputs. For multiple-layer networks, the layer number determines the superscript on the weight matrix, as shown in Figure 7. In the two-layer tansig/purelin network, the appropriate notation is used. This network can be used as to approximate general functions. It can approximate any function with a finite number of discontinuities arbitrarily well, given

- 250 sufficient neurons in the hidden layer.
- 251

252

Figure 7 Multilayer networks with weight matrix.

In this section, the final weights of each parameter have been calculated in order to study the effects of each 253 parameter on both safe and buckling temperatures. Note that the results can be calculated directly from 254 255 MATLAB without knowing weight matrix. However, it can be calculated only one by one. A weight matrix 256 can be obtained and used to create the function to help calculate the results automatically for parametric studies. Figure 8 shows the contour of safe and buckling temperatures considering the lateral stiffness from 257 258 ballast and sleeper and torsional resistance provided by fastening system. It is found that increasing 259 torsional resistance provided by the fastening system can help increase the safe temperature but there is no 260 influence on the buckling temperature. As for the lateral stiffness, increasing the displacement limit from 1mm to 2mm can significantly increase the buckling temperature since the lateral resistance with higher 261 displacement limit yields at higher lateral resistance force. 262

(a)	(b)
(c)	(d)

263

Figure 8 Safe (T_{min}) and buckling (T_{max}) temperatures.

264 Figure 9 presents the effects of track lateral misalignment on the safe and buckling temperatures. It is notable that, overall, buckling temperatures decrease as the misalignment amplitude increases. On the other 265 hand, the lateral misalignment has a very slight effect on safe temperature in comparison to buckling 266 267 temperature. Considering the unconstrained length and initial misalignment, the safe and buckling temperatures are compared in Figure 10. The overall results show that railway tracks are generally buckled 268 269 within the same ranges when the unconstrained length is larger than 18 m whereas railway tracks with 6 m and 12 m buckled with much higher temperature. It is clear from this figure that increasing misalignment 270 tends to shift the buckling mode from snap-through buckling to progressive buckling for all cases of 271 272 unconstrained length since the buckling temperatures are quite close to the safe temperatures. These results 273 agree well with the previous FE results [33, 34].

Figure 9 Effects of track lateral misalignment on the safe and buckling temperatures.

276

Figure 10 Effects of track unconstrained length on the safe and buckling temperatures.

²⁷⁴

278 **5 Conclusions**

- This paper is the first to establish a machine learning aided design for predicting buckling phenomena of ballasted railway tracks. It is well known that the buckling strength of ballasted track is affected by many factors that are not combined linearly. This shows that track buckling capacity calculation is complex and
- it may take large memory storage and time consumption to evaluate the buckling strength via the computer
- simulation. This paper considers four different ANN architectures with five input parameters and two output
- values while the number of hidden layers is set as 1. It is found that the accuracy of the model increases
- significantly when the number of hidden neurons is increased from 1 to 15. It is notably that only one hidden
- layer is sufficient to create a proper neural network as the R^2 values are already high. The best ANN model
- 287 consists of a hidden layer with 15 hidden neurons and is used to compare with the multilinear regression. 288 The optimal ANN presents the R^2 values of about 0.975 and 0.995 for safe temperature and buckling
- temperature, respectively, which are much higher than those obtained by the traditional regression method.
- 290 The predicted outputs can be used to estimate the buckling failure modes by calculating the difference
- between the buckling and safe temperatures. This paper will provide a pathway for improving the predictive
- 292 model of buckling phenomena of ballasted railway tracks. Therefore, the neural network model can be
- applied to help predict buckling failure mode for the complex track models in order to detect track buckling
- in summer.

295 6 Acknowledgements

296 The authors are sincerely grateful to European Commission for the financial sponsorship of the H2020-

- 297 MSCA-RISE Project No. 691135 "RISEN: Rail Infrastructure Systems Engineering Network," which
- enables a global research network that tackles the grand challenge of railway infrastructure resilience andadvanced sensing in extreme environments (www.risen2rail.eu) [38].

300 7 References

- I. S. Oslakovic, H. W. T. Maat, A. Hartmann, G. Dewulf, Risk Assessment of Climate Change
 Impacts on Railway Infrastructure, (2013).
 A. D. Quinn, A. Jack, S. Hodgkinson, E. J. S. Ferranti, J. Beckford, J. Dora, Rail Adapt: Adapting the
- A. D. Quinn, A. Jack, S. Hodgkinson, E. J. S. Ferranti, J. Beckford, J. Dora, Rail Adapt: Adapting the
 Railway for the Future, A Report for the International Union of Railways (UIC) (**2017)**.
- 3053.C. Ngamkhanong, S. Kaewunruen, B. J. A. Costa, State-of-the-Art Review of Railway Track306Resilience Monitoring, Infrastructures. 3(1) (**2018)** 3.
- S. S. N. Ahmad, N. K. Mandal, G. Chattopadhyay. "A Comparative Study of Track Buckling
 Parameters on Continuous Welded Rail." 26-28.
- 309 5. C. Esveld, *Modern Railway Track*. Vol. 385: MRT-productions Zaltbommel, Netherlands, 2001.
- 3106.A. Kish, On the Fundamentals of Track Lateral Resistance, American Railway Engineering and311Maintenance of Way Association (**2011)**.
- 312 7. C. Esveld. "Improved Knowledge of Cwr Track." 8-9, 1997.
- 3138.L. Ling, X. B. Xiao, Y. B. Cao, L. Wu, Z. Wen, X. S. Jin, Numerical Simulation of Dynamical314Derailment of High-Speed Train Using a 3d Train–Track Model. 2014.
- 3159.L. Ling, X. B. Xiao, X. S. Jin, Development of a Simulation Model for Dynamic Derailment Analysis316of High-Speed Trains, Acta Mechanica Sinica. 30(6) (**2014)** 860-75.
- 31710.S. Kaewunruen, Y. Wang, C. Ngamkhanong, Derailment-Resistant Performance of Modular318Composite Rail Track Slabs, Engineering Structures. 160 (**2018)** 1-11.
- 31911.M. Cuadrado, C. Zamorano, P. González, J. Nasarre, E. Romo, Analysis of Buckling in Dual-Gauge320Tracks, Proceedings of the Institution of Civil Engineers-Transport. 161 (**2008)** 177-84.
- I. Villalba, R. Insa, P. Salvador, P. Martinez, Methodology for Evaluating Thermal Track Buckling
 in Dual Gauge Tracks with Continuous Welded Rail, Proceedings of the Institution of Mechanical
 Engineers, Part F: Journal of Rail and Rapid Transit. 231(3) (2017) 269-79.

324	13.	G. Yang, M. A. Bradford, Thermal-Induced Buckling and Postbuckling Analysis of Continuous
325		Railway Tracks, International Journal of Solids and Structures. 97 (2016) 637-49.
326	14.	C. Ngamkhanong, C. M. Wey, S. Kaewunruen, Buckling Analysis of Interspersed Railway Tracks,
327		Appl. Sci. 10 (2020) 3091.
328	15.	A. Kish, G. Samavedam. "Track Buckling Prevention: Theory, Safety Concepts, and Applications."
329		John A. Volpe National Transportation Systems Center (US), 2013.
330	16.	A. Kish. "On the Fundamentals of Track Lateral Resistance." In Annual Conference. Minneapolis,
331		USA, 2011.
332	17.	G. Jing, P. Aela, Review of the Lateral Resistance of Ballasted Tracks, Proceedings of the
333		Institution of Mechanical Engineers, Part F: Journal of Rail and Rapid Transit. 234(8) (2020) 807-
334		20.
335	18.	Y. Guo, H. Fu, Y. Qian, V. Markine, G. Jing, Effect of Sleeper Bottom Texture on Lateral Resistance
336		with Discrete Element Modelling, Construction and Building Materials. 250 (2020).
337	19.	C. Ngamkhanong, B. Feng, E. Tutumluer, Y. M.A. Hashash, S. Kaewunruen, Evaluation of Lateral
338		Stability of Railway Tracks Due to Ballast Degradation, Construction and Building Materials. 278
339		(2021).
340	20.	A. D. Kerr, An Improved Analysis for Thermal Track Buckling, International Journal of Non-Linear
341		Mechanics. 15(2) (1980) 99-114.
342	21.	A. D. Kerr, Analysis of Thermal Track Buckling in the Lateral Plane, Acta Mechanica. 30(1-2)
343		(1978) 17-50.
344	22.	Y. S. Park, S. Lek, Chapter 7 - Artificial Neural Networks: Multilayer Perceptron for Ecological
345		Modeling. In Developments in Environmental Modelling, edited by Sven Erik Jørgensen, 123-40:
346		Elsevier, 2016.
347	23.	M. A. B K A, C. Ngamkhanong, Y. Wu, S. Kaewunruen, Recycled Aggregates Concrete
348		Compressive Strength Prediction Using Artificial Neural Networks (Anns), Infrastructures. 6(2)
349		
350	24.	X. Huang, J. Zhang, J. Sresakoolchai, S. Kaewunruen, Machine Learning Aided Design and
351	25	Prediction of Environmentally Friendly Rubberised Concrete, Sustainability. 13(4) (2021).
352 352	25.	N. T. DO, M. Gul, Estimations of Vertical Rall Bending Moments from Numerical Track Deflection
333 354		Transportation Engineering Dart A: Systems 147(2) (2021)
354	26	C Deroira Silva M S Dersch L P Edwards Quantification of the Effect of Train Type on
355	20.	Concrete Sleeper Pallact Process Lising a Support Condition Pack Calculator. Frontiors in Puilt
357		Environment 6 (2020) 214
358	27	L Sresakoolchai S Kaewunruen Detection and Severity Evaluation of Combined Rail Defects
359	27.	Using Deen Learning Vibration 4(2) (2021)
360	28.	M. C. Nakhaee, D. Hiemstra, M. Stoelinga, M. Noort, The Recent Applications of Machine
361	20.	Learning in Rail Track Maintenance: A Survey, Paper presented at the RSSRail 2019.
362	29.	W. Li, H. Chen, Y. Zhang, Y. Shi, Track Slab Crack Detection Based on Full Convolutional Neural
363		Network. Paper presented at the Journal of Physics: Conference Series 2021.
364	30.	H. Khaiehei, A. Ahmadi, I. Soleimanmeigouni, M. Haddadzade, A. Nissen, M. J. Latifi Jebelli.
365		Prediction of Track Geometry Degradation Using Artificial Neural Network: A Case Study,
366		International Journal of Rail Transportation (2021) .
367	31.	H. Alawad, S. Kaewunruen, M. An, A Deep Learning Approach Towards Railway Safety Risk
368		Assessment, IEEE Access. 8 (2020) 102811-32.
369	32.	H. Alawad, S. Kaewunruen, M. An, Learning from Accidents: Machine Learning for Safety at
370		Railway Stations, IEEE Access. 8 (2020) 633-48.

371	33.	C. Ngamkhanong, S. Kaewunruen, C. Baniotopoulos, Influences of Ballast Degradation on
372		Railway Track Buckling, Engineering Failure Analysis. 122 (2021) .
373	34.	C. Ngamkhanong, S. Kaewunruen, C. Baniotopoulos, Nonlinear Buckling Instabilities of
374		Interspersed Railway Tracks, Computers and Structures. 249 (2021).
375	35.	S. Kaewunruen, C. Ngamkhanong, J. Ng, Influence of Time-Dependent Material Degradation on
376		Life Cycle Serviceability of Interspersed Railway Tracks Due to Moving Train Loads, Engineering
377		Structures. 199 (2019) .
378	36.	U. Atici, Prediction of the Strength of Mineral Admixture Concrete Using Multivariable
379		Regression Analysis and an Artificial Neural Network, Expert Systems with Applications. 38(8)
380		(2011) 9609-18.
381	37.	C. Bilim, C. D. Atiş, H. Tanyildizi, O. Karahan, Predicting the Compressive Strength of Ground
382		Granulated Blast Furnace Slag Concrete Using Artificial Neural Network, Advances in Engineering
383		Software. 40(5) (2009) 334-40.
384	38.	S. Kaewunruen, J. M. Sussman, A. Matsumoto, Grand Challenges in Transportation and Transit
385		Systems, Frontiers in Built Environment. 2(4) (2016).