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Quantifying the hydrological implications of preand post-installation willowed engineered log jams in the Pennine Uplands, NW England

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1	Quantifying the hydrological implications of pre- and post-					
2	installation willowed engineered log jams in the Pennine Uplands,					
3	NW England					
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24 Highlights

- Willowed log jams (~dams) have been installed frequently to reduce flood risk.
- Few studies have assessed pre- and post- installation changes to watercourse flows.
- Discharge data shows an average 27.3% reduction on peaks, following installation.
- River-reach (0-130m) wildlife camera photos and levels confirm attenuation.
- Willowed log jams re-naturalise flows, locally alleviating floods and droughts.

31 - Abstract

32 Nature Based Solutions (NBS), including Natural Flood Management (NFM) schemes are 33 becoming an important component of many governmental and organisation responses to 34 increases in flood and aridity risk. NFM structures may take multiple forms to slow, store, 35 disconnect and filter distributed overland flow pathways within a catchment that coalesce to 36 generate a flood-wave downstream and runoff rather than infiltrate groundwaters. To date 37 few studies have conducted observations pre- and post-installation monitoring at river reachscales, despite widespread and frequent installation, to investigate the efficacy of willowed 38 39 engineered log jams (WELJs) interventions used in abating flood-flows, through backing-up flood-pulses with consequent reductions in downstream discharges. This paper examines 40 41 the efficiency, before and after installation of five 1 metre high WELJs incorporating 1,000 Bay willow (Salix pentandra) saplings supporting the dead horizontal timber, across a total of 42 130 linear metres spanning the floodplain of a decommissioned reservoir. One rain gauge, 43 two fixed point time-lapse wildlife cameras and three water level stations were installed: 44 45 upstream-of; within, and downstream-of all WELJs. The findings demonstrate a substantial 46 reduction is achieved for most events, with an average of 27.3% reduction in peak discharge being achieved post-installation. The time to peak is little impacted, however there is 47 demonstrable evidence of a longer and higher recessional limb to the events. These findings 48 49 quantify for the first time the role that WELJs can play in a move towards re-naturalisation of water level regimes, with lower peak water flows achieved, and waters released from the 50 51 river-reach more slowly. Furthermore, baseflow during dry periods is also elevated by 52 27.1%, offering greater resilience to dry periods and droughts. Consequently, over the river-53 reach scale (0-130 m), WELJs play an important role in alleviating flood and drought risk through suppressing flood peaks and increasing baseflow during low flows; steps towards 54 improved hydro-morphological quality overall. 55

56 Keywords

57 Natural flood management, flooding; hydrological connectivity; willow; engineered log jams.

- Quantifying the hydrological implications of pre- and post installation willowed engineered log jams in the Pennine Uplands,
 NW England
- 62 **1.** Introduction

63 1.1. Natural Flood Management in policy

64 Globally, 74% of disasters between 2001 - 2018 were water related, with the number of deaths exceeding 166,000 from floods and droughts, a trend which is set to rise (United 65 Nations World Water Development Report, 2020), and in the European Union annual flood 66 loses are projected to exceed €23 billion by 2050 (Jongman et al., 2014). Traditional flood 67 68 and drought protection measures have largely focused on engineered structures which are costly to construct and maintain (Thorne, 2014); however, increasingly natural approaches to 69 retaining water in the landscape are considered alongside traditional engineered approaches 70 (Pitt, 2008; Waylen, 2017). 71

Natural Flood Management (NFM) is a holistic approach based on an earth system 72 73 engineering principal that uses natural processes to slow the flow of water across 74 landscapes (Werritty, 2006). In 2019, at the UN Climate Action Summit, A Nature Based 75 Solutions (NBS) for Climate Manifesto was launched, supported by 70 governments, private 76 sector, and international organizations, and accompanied by nearly 200 initiatives of best 77 practice combating all water security needs (United Nations, 2019). The aspiration of NFM is 78 to create a holistic catchment wide network of interventions that produce single site 79 improvements that collectively reduce downstream flood risk alongside a range of additional 80 benefits. Whilst flood management has traditionally been the focus, the benefits of NFM are 81 much greater, what Barlow et al. (2014) calls 'multiple benefits', with improvements in 82 biodiversity (Cook et al., 2016), groundwater recharge (Hut et al., 2008), carbon sequestration (WWAP, 2018), public health (Postnote, 2016), water quality (Barber and 83 Quinn, 2012), and the provision of recreational areas. 84

85 1.2. Natural Flood Management in practice

86 In the UK, current and future flood risks are being compounded by a combination of climate 87 and land use change; with peak runoff flows being observed to increase at a rate of over 5% 88 per decade, while 10% of new homes are being built in areas of significant flood risk - land 89 with a \leq 1:100-year probability of flooding (Putro *et al.*, 2016; DEFRA, 2018; Blöschl *et al.*, 90 2019; Ministry of Housing, Communities and Local Government, 2020). Studies have 91 highlighted how some 12,200 km² of UK land is at risk of flooding, including 1 in 6 properties, 92 which in total implicates circa 5 million people (Hall et al., 2003; Environment Agency, 2009; 93 House of Commons, 2016). Climate change and land use scenarios predict that flood risk 94 will increase in real terms; in frequency, magnitude and effect (Committee on Climate 95 Change, 2012; House of Commons, 2016). A family of scenarios for: climate change impact; long-term increasing development on the floodplain (Committee on Climate Change, 2012); 96 97 and increasingly impermeable catchments will cumulatively result in more property exposure to flood risk from increasingly flashy watercourses, with impaired groundwater recharge (Hut 98 99 et al., 2008; Kendon et al., 2014; Putro et al., 2016). The Department for Environment, Food 100 and Rural Affairs' (DEFRA) 'Making Space for Water' (2004) approach has been trialled in several places on an ad hoc basis for some time (Burgess-Gamble et al., 2017; Nicholson et 101 al., 2020). In the UK 4,185 NFM assets have been created with woody leaky barriers make 102 103 up 69% of all measures (JBA Trust, 2020; DEFRA, 2021A). To-date, a dearth of preinstallation observations exist that would permit a pre- and post-installation assessment to 104 be undertaken (Arnott et al., 2018). Ellis et al., (2021) provide a useful empirical scale-based 105 assessment of NFM data to date, whilst Black et al., (2020) observes catchment-scale lags 106 of 2.6 - 7.3 hours where leaky woody structures and other measures occurred. As Leakey et 107 al., (2020:1) observe these "barriers have been implemented widely, there is still resistance 108 109 to their use at the scales required to impact significantly on flood risk, at least partially due to an evidence gap". Consequently, since modelling and evidence is the basis to much NFM 110 111 development, without a determination of the impact on observed flows and levels, which can

be used to calibrate models (Hankin *et al.*, 2016; 2017), many schemes may not progress.
This paper addresses this research and understanding gap, by demonstrating reach scale
level and discharge change, for both floods and low flows (see Hut *et al.*, 2008).

115 The data and analysis presented in this paper are based on a series five willowed log jam 116 structures installed in late 2019 in the Smithills Estate, near Bolton in North West England led by the Mersey Forest, Woodland Trust, Environment Agency and installed by specialist 117 118 contractors - Pownall Plant Ltd. Water harvesting and retention techniques have been used since 9,000 BC (Oweis et al., 2001), whilst human uses of willow pre-date stone age 119 technology (> 3,300 BC; Kuzovkina and Quigley, 2005). Yet no studies prima facie 120 document the effects of WELJs on flow, despite abundant distribution water tolerant willows 121 122 (Salix sp.) across arid and temperate regions of the world - totalling 450 species across all continents bar Antarctica (Zhen-Fu, 1987; Kuzovkina and Quigley, 2005). Consequently, the 123 evidence and approach documented is likely to have international application to similar 124 headwater reaches, where water retention by substrate are infeasible. No two catchments 125 126 are the same, presenting heterogeneity in form, pattern and process, which is problematic to 127 map and predict at finite scales under a reductionist approach, which may seek to specify the details of heterogeneity through spatial distributions (Sivapalan, 2018). By taking an 128 129 earth system science approach, which acknowledges commonality in hydro- eco- geo- pedo-130 logical processes across all catchments, the conceptual opportunity to apply the WELJ approach to first and second stream orders in similar climatic and geomorphological settings 131 is presented (*ibid*). 132

133

134 1.3. Natural Flood Management

Where practiced, NFM often seeks to address quick-flow/direct runoff propagation. NFM techniques are defined as the alteration, restoration of use of landscape features to spatially engineer measures to slow, store, disconnect and filter river and overland flows in sufficient volume to alleviate downstream flood risk (Wilkinson *et al.*, 2010; Burgess-Gamble *et al.*, 139 2017). NFM draws upon multiple sets of expertise including natural scientists, hydrologists, engineers, and social scientists, combined with knowledge from local communities (Bark et 140 al., 2021). Proponents of this holistic and often partnership-based approach advocate that 141 "These practices could be taken up more widely in the UK, and internationally, to manage 142 143 floods, droughts and pollution" (Quinn et al., 2016:1). Yet despite wide advocacy of the multifunction benefits to the natural and human worlds, Wingfield et al. (2019) note the lack of 144 widespread adoption could reflect a focus on research and resources aimed at increasing 145 146 the evidence base; a lengthy and complex goal, if NFM is not holistically applied at the 147 catchment-scale. Furthermore, the Agricultural Act of 2020 which is in its implementation phase via UK Environmental Land Management schemes (DEFRA, 2021B) will, in the fu-148 149 ture, allow funding of a wider-range of public ecosystem service actions, including NFM 150 (Holstead et al., 2014; Green Alliance, 2017).

151

152 **1.4.** The Runoff Attenuation Feature (RAF) approach

Natural flood management encompasses a gamut of measures, including tree planting, 153 peatland, agricultural soil and river restoration techniques. One approach focused on 154 addressing rapid rainfall-to-runoff responses, or flashiness, is through Runoff Attenuation 155 Features (RAFs) (after Nicholson et al., 2012). The hydrological premise is that, if a sufficient 156 number of features are deployed around a river catchment, targeting multiple sources and 157 158 pathways of quick-flow, then runoff can be attenuated at numerous spatial-scales, diffusing and retaining the tributary flood-pulses, before they coalesce to create peak flow 159 160 synchronicities, and hence, flood the urban receptor (Wilkinson et al., 2010; Nicholson et al., 2012; Figure 1 in Norbury et al., 2019). Whilst the wider gamut of NFM techniques offer 161 162 multi-functional benefits (e.g., Burgess-Gamble et al., 2017), the ability to indicatively quantify their efficacy in business case terms is often problematic (Hankin et al., 2017). 163 Many communities at flood risk are covered by hydraulic models which can be assessed to 164 165 determine return period spill volume over bank into floodplain and dwellings (Norbury et al.,

166 2019). This floodplain spill volume can then become a catchment attenuation requirement 167 above the community at flood risk, and hence, RAFs which can be monitored readily and 168 volumetrically calculated, individually or as a collectively across a catchment, and frequently 169 present a best available technique to alleviate risk (Nicholson *et al.*, 2012; Hankin *et al.*, 170 2017:4; Norbury *et al.*, 2019).

Areas characterised by intense drainage density, over-grazing and high livestock densities, 171 172 soil compaction, and steepness of slope, are commonplace in many uplands, sites prime for RAFs (Bracken and Croke 2007; Wilkinson et al., 2010; Marshall et al., 2014). RAFs are 173 interventions that alter pluvial and fluvial pathways; physically restricting the passing-forward 174 of downstream flood-flow, and hence, reduce peak runoff and velocity, but convey baseflow 175 176 (Wilkinson et al., 2010; Nicholson et al., 2012). RAFs can include drained earth and stone bunds (~embankments), ditches often perpendicular to the watercourse, attenuation ponds 177 and scrapes, excavations of pluvial hollows or floodplains, leaky barriers, including live 178 willowed log jams or sawn treated timber barriers and woodland planting (Wilkinson et al., 179 180 2010; Nicholson et al., 2012; Burgess-Gamble et al., 2017). The physical backing-up of 181 water by these measures, for temporary periods, often 0 - 80 hrs (storm dependant), also results in increased infiltration and probable groundwater recharge (Hut et al., 2008; 182 Wilkinson et al., 2010; Wainwright et al., 2011; Norbury et al., 2020). RAFS fall into two 183 184 categories: offline, which attenuate water off the floodplain and online which holds water on the floodplain (Nicholson et al., 2012). 185

One of the most cost-effective and frequently used measures is the introduction of willowed log jams in the upper and middle reaches of a catchment, such interventions intercept propagating flood-waves heading down-channel and attenuate floodwater behind the dam on floodplains (Burgess-Gamble *et al.*, 2017; Muhawenimana *et al.*, 2021). Willowed engineered log jams often consist of tree trunks, 2.5 times stream-width keyed into the riverbanks to allow sufficient passage of base flow through the obstruction, then during highflows the logs trap and attenuate water behind the log-jam. To avoid bypass, willow-woven trunks can be planted across the floodplain perpendicular to flow. Planting behind the logs
makes the structure a living bio-filter, resilient to movement and increases structural stability
and longevity.

196

197 2.0. Methodology

198 **2.1.** Study Site: Two Lads and the Woodland Trusts Smithills Estate, Bolton

Recognising the need to trial NFM, the UK Government launched a fund for bids to deliver 199 £15m of NFM from 2017 – 2021 (Wentworth et al., 2020). Locally, the Environment Agency 200 opened-up a £1m competition, and following a bid from the Woodland Trust, the Smithills 201 202 Estate was successful in a partnership project with the Environment Agency to deliver a 203 NFM project starting in 2019. The focus was predominantly on capital interventions over detailed monitoring. Two Lads was one of 11 locations selected to site 44 NFM installations 204 205 on the Woodland Trust's Smithills Estate, and it is part of a wider programme of NFM 206 interventions regionally. The study site is a headwater stream of Dean Brook, a tributary of the River Irwell which discharges to the Mersey (Figure 1, 2 and Video SM1). Dean Brook is 207 situated above Smithills, Bolton, in the northwest of England, and rises at approximately 456 208 209 meters on the peatlands of Winter Hill. The site consists of peat sitting on a course-grained 210 feldspathic sandstone, except in incised sections which have an alluvium bed. At its rural source, Dean Brook flows off the West Pennine Moor Site of Special Scientific Interest, via 211 an extensive grip network, into the Clough Woodlands and Dean Brook before reaching 212 Smithills, a suburb of Bolton, Greater Manchester. The settlement at Smithills is at flood risk, 213 with 12,300 m³ of water predicted to spill into the floodplain during a 1 in 100-year event, 214 215 potentially affecting 53 properties (Figure SM2; See Hankin et al., 2017:4 and Norbury et al., 2019 for flood volume appraisal techniques). Smithills Estate is being used to trial a range of 216 landscape-scale restoration techniques, including re-wilding, led by the Woodland Trust see 217 Bridges et al., (2021:180) for further background. 218

219 The Dean Brook catchment has a long legacy of industrialisation, with several abandoned mine shafts (predominantly coal), mill ponds/impoundment structures to support the upland 220 early industry (17th and 18th century brick works) and subsequent reservoirs (three, of which 221 two have been decommissioned) constructed to support the emerging textile industry and 222 associated bleach works in the lower valley in the 19th century. During the early 20th century 223 the area was extensively drained, with the industrial structures of previous centuries 224 abandoned, however this has left a legacy of hydrological manipulation, with many channels 225 straightened and canalised with a focus on increased drainage efficiency downstream. The 226 catchment provides the spatial unit (0.72 km² upstream of WELJ5) and offers an opportunity 227 228 to deliver innovative ways of alleviating flooding through NFM, based on flood modelling of the area. Five willowed engineered log jam (WELJ) interventions were trialled at the Two 229 230 Lads site, situated in a former reservoir bed, which was last active in the 1840s adjacent to 231 the Hole Bottom 'Kiln' and 'Hall', situated in the NW corner of Figure 1A.

232

233 2.2. Pre-installation monitoring: apparatus and methodology

A decommissioned reservoir presents an opportune site to attenuate flood-flow, as they are 234 often flat and wide, hence relatively short structures may retain a large volume of water 235 compared to a steeply incised landscape-setting (See Case Study 17 in Burgess-Gamble et 236 al., 2017 and Norbury et al., 2020 for example efficacy; Video SM1 and 2). On 8th August 237 238 2019 three water level stations (WLS1-3) were installed across five willowed engineered log jams (WELJ001-005). WLS1 was situated 85m upstream of the final WELJ (005) and 239 240 beyond the attenuation area, WLS2 was installed within the area of inundation behind the second to last log jam (WELJ002) and WLS3 was placed 100 linear metres downstream of 241 242 all log jams (Figure 1A). An EML ARG 3 rain-gauge was discretely installed 90m to the NE of the WELJs (not visible from the footpath that runs adject to the site), a Tempcon HOBO 243 U20L-04 atmospheric pressure logger was placed adjacent to WLS3 and two Crenova 244 wildlife cameras were installed looking downstream over the log jams WELJ001-4 and the 245

246 second provides a side view of WELJ002 (Figure 1A). Typical level accuracy is at ±0.1% or 4 mm and for rainfall it is above the 99% confidence level up to 120mm hr⁻¹. Together these 247 instruments served to provide simultaneous 15-minute precipitation (mm) and water level 248 (m) data along with hourly time-lapsed photos of the willowed log jams. HOBO pressure 249 250 loggers were hung in a perforated stilling well situated at the base of the entrenched channels; an atmospheric pressure logger served to compensate the water level loggers 251 using Tempcons Hoboware software. Additional precipitation data has been accessed from 252 253 the Environment Agency (EA), with the nearest meteorological station situated at Lower Rivington (Stn. Num. 569723, 53°36'15.5"N, 2°33'32.2"W), annual average precipitation 254 1,174.5 mm (1981-2010), <5km away from the Two Lads site and provides quality control 255 and supplemental data. The EA rain-gauge has been used to infill missing precipitation data 256 arising from a technical fault on the Two Lads rain-gauge (28th Oct. - 7th Dec. 2019; Figure 257 258 1A).

259

260 2.3. Willowed engineered log jam interventions

Specialist contractors started and completed the five willowed log jam build between August 261 21st – 30th 2019, with a total length of 130 linear metres (Figure 1, 2 and Video SM1), offering 262 a total attenuation capacity of ~3,000 m³ across the suite of WELJs, with increasing capacity 263 264 moving downstream. Figure 1B provides a schematisation of the WELJs, these are 265 horizontal felled timbers, staked front and back every two linear metres with strainer posts (2 = 103) bay willow (Salix pentandra) planted at 8 per linear metre (Σ = 1,040), to create a 266 living thicket of live shrubs. In total, 54 tonnes of locally sustainably harvested timber was 267 used in the WELJs, approximately 130 stems of whole tree at 9m L and 300mm maximum 268 269 diameter. The timber was a mixture of native, European and North American softwoods including Scots- (Pinus sylvestris), Logepole- (Pinus contorta) and Corsican pine (Pinus 270 *nigra*) derived from thinning from Burnt Edge legacy plantation inappropriately located on dry 271 heathland (53°36'26"N, 002°30'02"W). Since not all log jams are created equal (Dixon 272

273 2015A, 2015B); WELJs advance the longevity and structural performance compared to conventional log jams. The use of willow to create a thicket both, holds-up the horizontal 274 deadwoods which are set to heavily decayed by 2029 (Burgess-Gamble et al., 2017; Dixon 275 et al., 2018; Thomas and Nisbet, 2020) and provides immediate resource to add to the 276 277 horizontal timber (Figure 1B). In stacking timbers, as the horizontals saturate and descend into the floodplain, those horizontals on top in the stack provide a continued vertical barrier 278 to attenuate a head of water. A new planation of broadleaves nearby, can be sustainably 279 280 harvested to do this, also (Figure 1A).

281

282 2.4. Post installation monitoring

A difference in minimum baseflow between WLS1 and WLS3 was identified during the pre-283 instrumental period (08-29/08/2019; ~0.165 m³s⁻¹), which is replicated post-installation with a 284 longer observation window (30/08/2019-02/10/2020; ~0.15 m³s⁻¹), a negligible difference 285 between the two. During analysis WLS3^{adj} is used to represent the WLS3 measurements 286 with baseflow removed (0.165 m³s⁻¹). WLS3 is situated within a bedrock gorge, ~100m 287 downstream of the last WELJ, at 344 mAOD, this represents a fall in elevation of 15 m from 288 WLS1, with both surface and subsurface flows channelled into the bedrock gorge from the 289 sub-catchment. During intense precipitation events additional lateral surface flow channelled 290 291 along the adjacent footpath enters downstream of the installations, but upstream of WLS3, this has only been observed during high flows >0.5 m³s⁻¹ @WLS1 arising from intense 292 precipitation (>4mm/15min) and/or during snowmelt events; spot gauging during high lateral 293 flows (14/01/2021) determined an addition of ~0.2 m³s⁻¹. During low and normal conditions 294 295 no lateral inflow is presented, this additional lateral inflow was not identified during site 296 selection or during the early instrumentation phase (Figure 1 and Video SM4).

297

299 **3.0. Results**

300 **3.1. Rainfall-runoff analysis**

301 Precipitation and water level data from the three pressure transducers (WLS1-3; Figure 3A -D) demonstrate that whilst the pre-installation period was relatively short, a range of 302 precipitation events of varying magnitude, frequency and duration were captured, with 303 comparable events pre- and post-installation of the WELJs. The reduction in precipitation in 304 late-March 2020 to mid-June 2020 is captured within the water level stations at the three 305 sites, with little flow reaching the WELJs during the dry months March-June (Figure 3B-D), 306 demonstrating a typical hydrological response for an upland peat-moorland. The annual 307 308 precipitation pattern in 2020 reflects the long-term pattern (1971-2010), with February to July average monthly precipitation <100 mm and all other months receiving >100 mm. Notably 309 April has the lowest average monthly precipitation (74.3 mm), whereas October has the 310 highest (129.1 mm). 311

Analysis of comparable isolated events, where precipitation and discharges exceeded 7.5 312 mm hr⁻¹ and 0.5 m³s⁻¹, indicates a reduction in water level at WLS3 relative to WLS1 313 (Figures 4A-H), with greater reductions in water level for higher magnitude precipitation 314 315 events (Figure 4C-D, G-H). Comparison of events under comparable catchment antecedent conditions demonstrates a similar pattern (10:00-22:00 16/08/2019 compared to 05:00-17:00 316 09/10/2019), with a reduction in water level and discharge achieved at WLS3 post-WELJ 317 installation relative to pre-installation (Figure 3; Figure 4C). Lateral inflow was observed 318 (Figure 1A; Video SM4) and spot gauging estimated at 0.2 m³s⁻¹, an equivalent the 27% 319 higher flow noted during Storm Ciara. The lag time pre- and post-WELJ installation is 320 typically around 135 minutes from peak precipitation to peak discharge at WLS1, with no 321 322 discernible difference in the timing peak precipitation and peak flow pre- and post-WELJ 323 installation. The efficiency of the WELJs during the two events are demonstrated by Figure 4D and 4H (also see supplementary material SM3), which demonstrate the elevated water 324 325 levels present in WELJ002 throughout the period of analysis, with a higher base water level retained in the installation during the winter months, with a return to pre-installation levels only during dry periods (Figure 4C).

328

329 3.2. Comparable pre- and post- engineered log jam hydrographs

In assessing the hydrological efficiency of the five willowed WELJs, comparative analysis of 330 331 pre- and post- installation discharges between WLS1 and WLS3 are undertaken (Figure 5A). A demonstrable difference is achieved post-WELJ installation, with a reduction in discharge 332 at WLS3 for most events, the reduction is relatively stable above 0.6 m³s⁻¹ (WLS3), with no 333 discernible reduction in capacity or attenuation effect for the events captured irrespective of 334 335 volume, with increasing storage above 0.45 m post WELJ installation (WLS3; Figure 4B-C). Higher baseflow (level) at WLS3 is achieved for low flows (<0.1 m3s⁻¹ @WLS1) post-WELJ 336 installation, a previously poorly documented benefit of NFM structures. Further analysis of 337 these changes in low baseflow (inflows of <0.1 m) show an increase of ~0.03 m post-WELJ 338 installation at WLS3 (Figure 5D). 339

Analysis of a series of precipitation events (>7.5 mm hr⁻¹) and associated peak discharges at 340 WLS1 (>0.5 m³s⁻¹) and WLS3 pre- and post- installation identifies an average reduction in 341 peak discharge of 27.3% across the river-reach at a range of event sizes (Table 1). Pre-342 installation events across the river-reach witness percentage change differences between a 343 10.5-102.6% increase, with an average of +42.1%; whereas post-installation sees a 344 percentage change of -11.4-124.8%, with an average of +14.8% (Table 1; Figure 6). The 345 reduction of 27.3% typically equates to a reduction of ~0.2 m³s⁻¹ on peak flows at WLS3^{adj} 346 (Figure 6). 347

348

349 3.3. February 2020 flooding: storm Ciara and Dennis hydrography

350 The period of analysis captures several precipitation events and subsequent run-off 351 responses for both pre- and post-WELJ installation, including the notable storms Ciara (8-

10th Feb. 2020) and Dennis (15-16th Feb. 2020) (Figure 7). Storms Ciara and Dennis brought 352 intense and prolonged rainfall to northern England in February 2020, with North West 353 354 England experiencing 321% on its 1981-2010 long-term February average and recording the wettest February on record since 1910 (Sefton et al., 2020; Simon et al., 2020). Three 355 356 people died in storm-related incidents with 3,000 properties flooded, more flood warnings and alerts were issued across the UK within a 24hr (16th Feb.) period since records began 357 (2006-present; (*ibid*). Close inspection of storm Ciara reveals a double rainfall peak, which 358 coalesced into a single peak in water level/discharge (Figure 7A). The hydrograph structures 359 through the log jam structures (WELJ002, WLS2) and after the log jams (WLS3^{adj}) show a 360 delaying in peak and attenuation of the flow against the inflow (WLS1, Figure 7 A-B). No 361 identifiable reduction in peak discharge is achieved during storm Ciara (percentage change 362 363 increase of 27%), in part arising from lateral inflow along the footpath during the intense precipitation phase (see supplementary material SM4), which exacerbates peak discharge at 364 WLS3 relative to discharges at WLS1 and WLS2 (Table 1). Elevated discharges are 365 identified in the recessional limb, with WLS3^{adj} demonstrating higher discharges compared to 366 WLS1, with the shape of the recessional limb reflecting that of the water level recorded at 367 368 WLS2. However, it is notable that the water level in WLS2 does increase and the photographic evidence indicates water retention was occurring during storm Ciara, even if no 369 370 discernible reduction in peak flow is documented at WLS3.

The subsequent storm Dennis reveals a more-prolonged less-intense precipitation event, with peak discharge at WLS3 being reduced by ~0.2 m³s-1, with a percentage change decrease of -4.3% (WLS1-WLS3^{adj}), with demonstrable storage within WELJ002 (Table 1; Figure 7B). Water levels in the log jam reservoir reveal attenuation and a release of flood waters after the peak has passed, with WLS3^{adj} initially below WLS1 on the recessional limb (Figure 7A).

The different hydrograph responses to the two events suggest that the WELJs had a greater attenuation role during storm Dennis compared to Ciara, likely reflecting the lower intensity more prolonged nature of the precipitation, though for both events the peak was reduced with storage and attenuation evident in WELJ002 (Figure 7). Whilst the cameras captured both events, images are of relatively poor quality because of poor visibility at the site, with storm Dennis floodwaters peaking at night. With continued monitoring a larger sample of higher-magnitude events will improve understanding of peak flow through the WELJs and provide a clearer depiction of the reductions afforded during peak flows.

385

386 **4.0. Discussion**

387 4.1. Reach-scale flow regime change

388 At the Two Lads site, the WELJs result in an average peak level reduction of 27.3% against the pre-installation peaks (Figure 3 and 5). Pre-installation, the channel was entrenched, 389 with dominant processes being narrowing and degradation, with low biotic interaction and 390 high erosion resistance - a stage two (channelized) or Rosgen A channel present (after 391 392 Rosgen, 1996; Cluer and Thorne, 2014). These channel types are sometimes so-called "firehose" channels, since few fluvial forms exist to slow the flow of passing waters. The 393 installation of the WELJs induced disturbance to the hydromorphology, and with it 394 395 ameliorated the flood propagating elements over the reach. The introduction of a physical 396 barrier has resulted in greater trapping of flood waters, where the flow exceedance of porosity and orifice space in the barrier results in backing-up and attenuation (Figure 2 and 397 Video SM1). Trapping of peak flood waves reduces the passage of the peak discharges 398 through the reach (Figure 3). The wildlife camera photos show filling and emptying of the 399 400 structures, and since Q = V*A (Q=discharge rate, A=area, V=velocity), a velocity rate change 401 can be observed meaning that the reduction in peak discharge passing through the reach 402 can be established causally, rather than associatively. This is further reaffirmed by the 403 changes longitudinally presented in the data from the stations, from upstream, within and 404 downstream of the WELJs (Figure 3 and 4). All conversions of water level to discharges are

405 based on ratings derived from repeat spot gauging at sites WLS1 and WLS3, with all level
406 data (pre- and post-instillation) converted to discharge.

WELJs enable assisted natural recovery (Burgess-Gamble *et al.*, 2017) of the reach with consequent process change discerned from time-lapse photos, from degradation and narrowing to widening and aggradation, which in time will lead to an anastomosing wet woodland (stage a stage 0 channel) as the WELJs separate single channel belts into multiple channels (Figure 2 and Video SM1; Dixon, 2015A, 2015B; Burgess-Gamble *et al.*, 2017; Dixon *et al.*, 2018; Norbury *et al.*, 2020).

413 To-date, the findings presented of reduced flood peak during events at all scales (once the impact of the lateral inflow is accounted for at WLS3) would appear prima facie contrary to 414 the proposition by Dadson et al. (2017) and Wilby and Dadson (2020) that NFM is only 415 operable during their so-called 'nuisance' flooding and would be overwhelmed during 416 417 extreme flows (Figures 1, SM2 and SM3; Video SM4). The Two Lads WELJs have undergone storm Ciara, Dennis and Christoph, with the wildlife camera (Figure 2 and Video 418 SM1) and level stations (Figure 3 and 7) showing no overwhelming - with no evidence of 419 420 WELJ overtopping.

As a comparison to reach-scale discharge reductions presented here, Dixon et al. (2016) 421 determined that restoring riparian forest cover over 20-40% of catchment area reduced flood 422 peak magnitude by up to 19%, yet restoration can take 25+ years to introduce large woody 423 debris into the channel sufficient to effect runoff rates. The physical morphology of the 424 channel will evolve to a more naturalised state, but the physical properties of the WELJ 425 426 structure will adjust through time, likely increasing flood-flow trapping efficiency (Section 2.3). The growth of the willow, and coppicing of it, will lead to increase trunk diameter over 427 428 the 1,080 whips, plus self-seeding, will increase the WELJ blockage and hydraulic roughness, particularly during summer foliation. This is predicted to enhance the trapping of 429 peak flows and prolong the recessional limb which is already notable in the data (Figures 3, 430 4 and 7). The increase in baseflow during low flow/drought events is nominal (27.1%; ~0.05 431

432 m³s⁻¹). However, a baseflow increase may have important implications for the local 433 hydroecology, suggesting woody structures such as those installed at Two Lads may have 434 an important role in landscape wetting, supporting the findings by Wilson *et al.*, (2011) who 435 have identified similar responses following drain blocking of peatlands in upland mid-Wales. 436 The absence of a discernible change in time to peak across the site may simply be reflective 437 of site size (130 m between WLS1 and WLS3, as such no clearly definable change is 438 achieved and flood events are relatively small and responsive 'flashy' events.

439

440 **4.2. Future WELJ research agendas**

Greater understanding of how the combination of the five WELJs are hydraulically interacting 441 would be advantageous, with results aggregating the efficiency of the structures on flood 442 443 peak attenuation. High-resolution repeat topographic surveys (e.g., Spreitzer et al., 2019), physical experiments (e.g., Follett et al., 2019; Follett and Wilson, 2020; Muhawenimana et 444 al., 2021) and numerical modelling studies (e.g., Boothroyd et al., 2016; Xu and Liu, 2017) 445 could further improve understanding of flow-structure interactions and sediment dynamics 446 associated with willowed engineered log jams and represent opportunities for further 447 enhanced understanding at the site. The site at Two Lads was not intended or designed as a 448 demonstration site, rather an opportunity was grasped to instrument the site prior to the 449 installation of the working structures. 450

The exact impact of WELJs would be sensitive to site specific context, however the findings identified within this study are comparable (23-50% reduction) to those considering natural woody dams created by beavers (Puttock *et al.*, 2020), without the challenges that reintroduction brings (Auster *et al.*, 2019). The potential role of WELJs in mimicking natural processes is considerable and could be an important tool in catchment-based flood risk management that combines both hard engineering and NFM interventions as advocated by Hewett *et al* (2020). This research provides valuable additional information to the evidence 458 base for NFM adoption; however, as Wingfield *et al.* (2019) note we should continue to 459 embrace such holistic measures in developing more resilient natural systems.

460 Pre-installation monitoring is often challenging but crucial in providing an evidence-led 461 approach, as recognised by CaBA (2017); as funding schemes used to support NFM 462 installation are varied and often have short timescales with limited funding assigned to preor post- installation monitoring (Robins et al., 2017). In undertaking this research on a 463 464 working site, it is evident that a longer pre-installation dataset would be optimal, however the timescales between funding being received, permissions of work gained, and installation to 465 begin were short, therefore a longer window was unavailable, this continues to represent 466 challenges for data acquisition. 467

Whilst this study used Bay Willow *(Salix pentandra) within the WELJ,* a species considered native across much of northern Europe and Asia that favours damp environments, and therefore are ideal for use in living WELJ, alternative species could be used to achieve similar impacts (see Zhen-Fu, 1987; Kuzovkina and Quigley, 2005).

472

473 **4.3. Strategic flood risk alleviation**

474 Understanding the efficacy of WELJs in abating flood-flow at the reach scale is important, since to alleviate flood risk in Smithills for the 1 in 100-year event, flows over >13 m³s⁻¹ 475 require retention in the upper catchment, corresponding to 12,300 m³ of water being 476 removed from the flood hydrograph peak, a depth reduction of 910 mm; representing 5% of 477 478 the total storm discharge (Figure SM2; Hankin et al., 2017; Norbury et al., 2019). This is equivalent to a 19% reduction of the peak water level and hence if NFM can be exercised at 479 sufficient scale it is hypothesized that the 1:100 year risk could be alleviated. At Smithills, the 480 interventions at Two Lads and the other 25 locations equate to that target volume of 12, 300 481 m³. Furthermore 65 Ha of woodland planting (130,000 trees), 60% of the river catchment, 482 are also ongoing on the Estate. Dixon et al. (2016) predict that for a 20-40% catchment 483

riparian woodland uplift, a 19% catchment outlet level reduction. In combination, the downstream flood risk reduction at Smithills should be tangible. As a comparison, the Holnicote NFM project which installed 41 log jams, 5 attenuation bunds, 5 ha of woodland, 5 fields of arable reversion, wet woodland and pond restoration in the 40ha catchment experienced a 10% reduction in flood peak during a 1 in 75 year event – the December 2013 floods – with none of the 98 at risk properties being flooded (Case Study 20 in Burgess-Gamble *et al.*, 2017).

491 Along with the NFM distributed across the catchment as shown in Figure SM2 further works 492 include peatland restoration on the headwaters including circa 100 stone dams, 50 peat bunds, 200 linear metres of reprofiling and surface contour bunds over 70,000 square 493 494 metres of the catchment (Gresty, 2020; see https://www.moorsforthefuture.org.uk/). Moorland restoration is noted to delay stormflow and reduce peak flows, with Shuttleworth et 495 al. (2019) observing delays of 106% and reductions of 27%. Consequently, the inflows to 496 Two Lads, particularly at WLS1, are predicted to be less flashy. Future research is urgently 497 498 needed for further catchment outlet monitoring, particularly as joined-up catchment-scale 499 restoration projects like this are uncommon. The data and findings on WELJ level induced reach changes will contribute to 1 and 2 dimensional hydraulic models, in particular unit 500 development and structural representation, which are used to predict flood benefits that 501 502 often underpin flood scheme prefeasibility and options assessment (see Hewett et al., 2020; 503 Leakey et al., 2020).

504

505 **5.0. Conclusion**

This research demonstrably identifies a 27.3% reduction in the average peak flow, with retention achieved in the WELJ structures (Figure 3). During storms Ciara and Dennis in February 2020, a comparable discharge at WLS1 to WLS3^{adj}, is achieved during storm Ciara accounting for baseflow and lateral inflows and during storm Dennis a reduction of 4.3% in peak flow was achieved between WLS1 and WLS3^{adj}. Whilst these present modest 511 reductions in peak flow, they represent a small sample, further high-magnitude events will 512 enhance understanding of WELJ capacity to reduce peak flows during high magnitude 513 events. Generally, discharges between precipitation events are increased with installations 514 slowly releasing waters long after flood-waves have passed, suggesting WELJs can play a role in increased water residence. The impact of these five WELS is more than holding water 515 516 in the landscape for longer, with the effect of more naturalised flow regimes, slower more sluggish water over flashier flows and reduced flood peaks. Together the findings 517 demonstrate the role these measures can play in flood and drought alleviation objectives as 518 519 guided by legislation.

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535 **Contributions**

Michael Norbury was the project manager, led project design and contributed to data 536 analysis and writing. Hazel Phillips undertook installation monitoring and data analysis and 537 contributed to writing. Neil Macdonald led the writing and contributed to field and data 538 539 analysis and project design. David Brown secured funding and contributed to project design, data analysis and writing. Richard Boothroyd undertook field monitoring and UAV 540 site work and contributed to writing. Catherine Wilson, Paul Quinn and Dave Shaw 541 supported in developing the paper. Wilson is undertaking further modelling of the 542 interventions, Quinn has visited site and provided independent quality assurance to the 543 544 Environment Agency and Shaw is a Trustee of the Community Forest which has supported this project and many others like it. 545

547 Conflicts of Interests

548 There are no conflicts of interest.

549 Data Access

550 The data from the site is continuing to be collected. As part of the funding requirements,

551 data will be uploaded onto the NERC repository once a substantial volume is 552 collected/completed. Earlier access may be gained through contacting Hazel Phillips.

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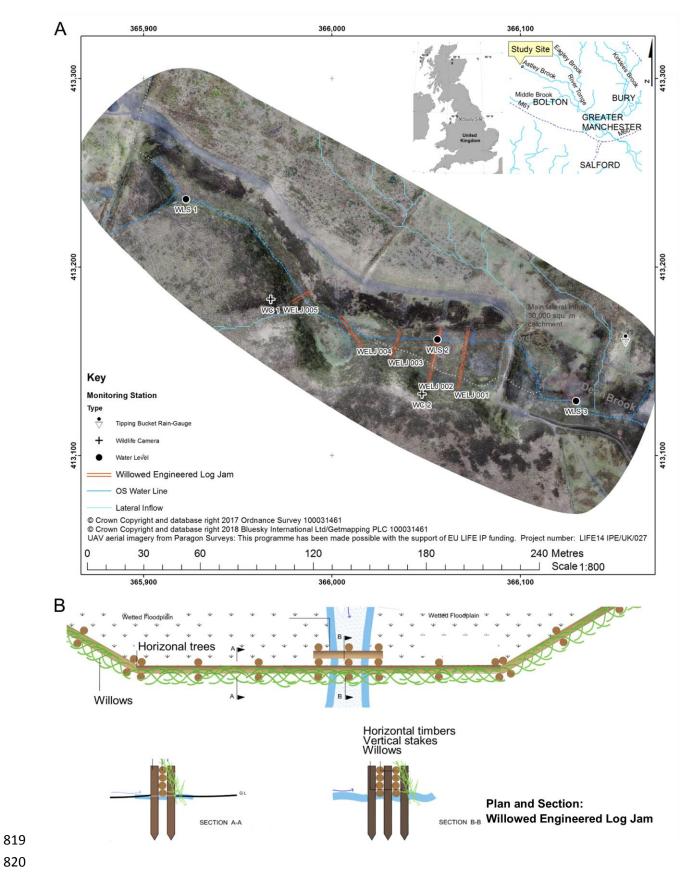
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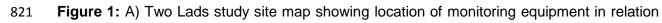
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- to WELJs; B) engineering schematic cross section of the WELJ.
- 823



Figure 2: A) Two Lads Study Site Photographs Prior to WELJ installation; B) the site follow-

825 ing WELJ installation

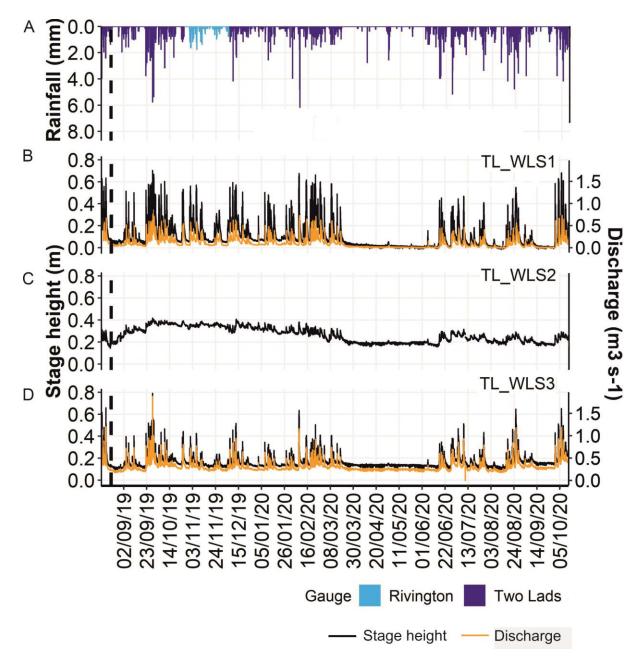


Figure 3: A) Daily precipitation at the Two Lads rain gauge and Rivington meteorological
station; B) continuous water levels for WLS1 (08/08/2019 - 02/10/2020); C) WLS
(08/08/2019 - 02/10/2020); and, D) WLS3 (08/08/2019 - 02/10/2020). The dashed vertical
line marks the installation of the five WELJ (19-30 August 2019).

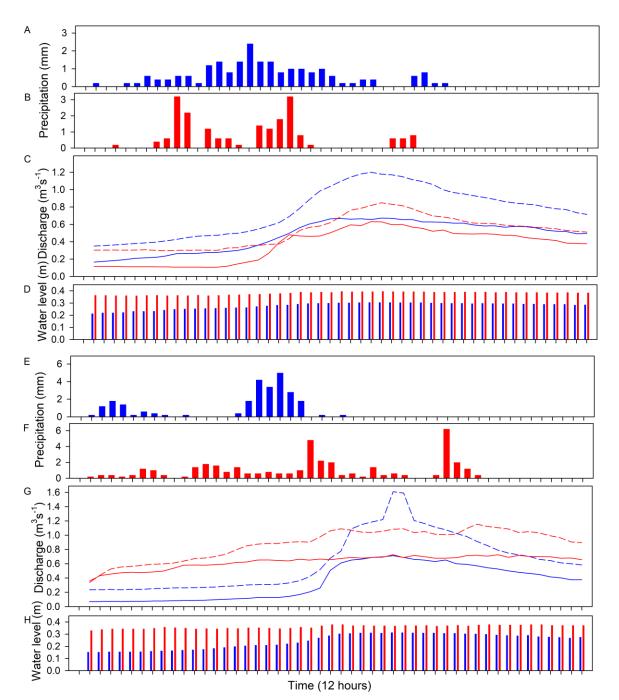


Figure 4: Comparison of similar magnitude pre- and post-WELJ installation 12-hour events,

A) precipitation (10:00-22:00 16/08/2019, blue), B) precipitation 05:00-17:00 09/10/2019,

- red), C) respective hydrographs at WLS1 (solid) and WLS3^{adj} (dashed) for the two events (a-
- b), D) comparative water levels in WELJ002 from WLS2 for each event; E) precipitation
- 842 (05:00-17:00 09/08/2019, blue), F) precipitation 02:00-14:00 02/09/2020, red), G) respective
- 843 hydrographs at WLS1 (solid) and WLS3^{adj} (dashed) for the two events, and H) comparative

844 water levels in WELJ002 from WLS2 for each event (E-F).

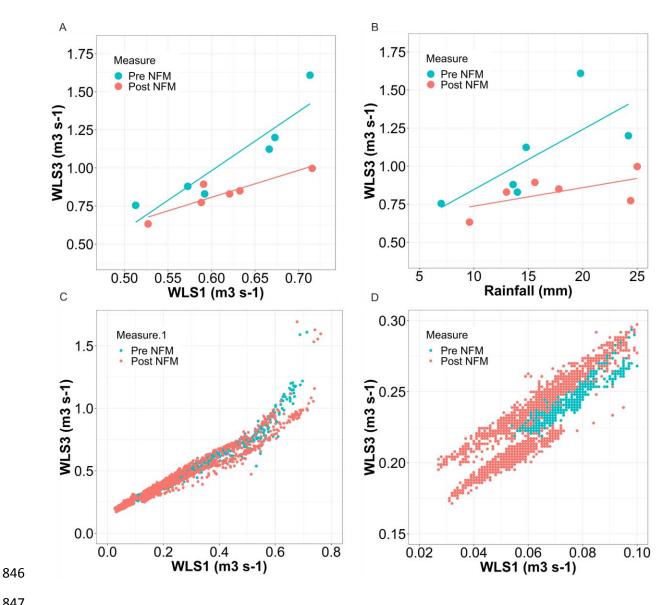
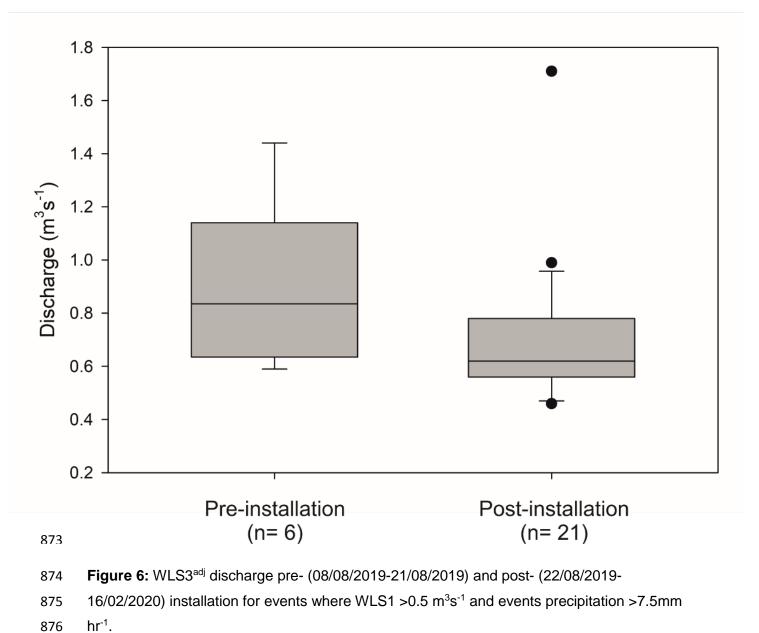
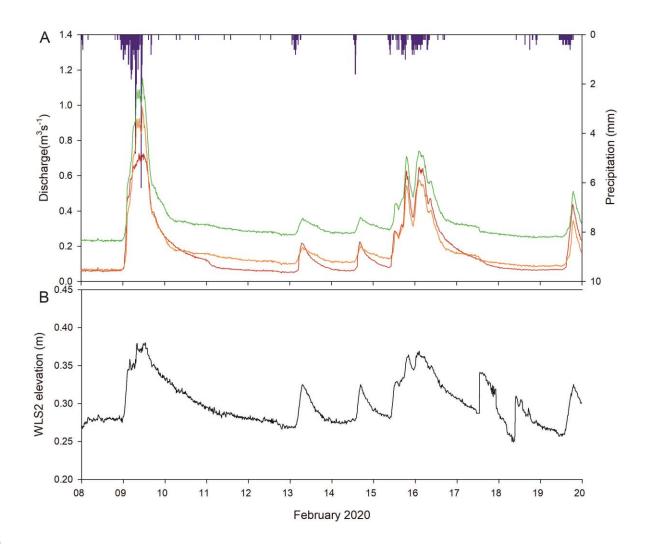


Figure 5: A) Peak inflow (WLS1) compared to peak outflow (WLS3) for comparable precipitation events (>10 mm per 0.25 hours) pre- (08/08/2019 - 18/08/2019) and post-WELJ installation (30/09/2020 - 10/10/2020); B) Total rainfall and peak discharge at WLS3 for comparable rainfall events pre- and-post WELJ installation; C); pre- and post-discharge relationship between inflow (WLS1) and outflow (WLS3); and, D) comparison of baseflow events (<0.1 m³s⁻¹ @WLS1) between WLS1 and WLS3 for pre- and post-WELJ installation.



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879 Figure 7: A) Precipitation (blue) and discharges at WLS1 (red) and WLS3 (green)

respectively for storms Ciara and Dennis during February 2020 (15min resolution), WLS3^{adj.}

(where minimum baseflow is removed 0.165 m^3s^{-1} ; gold) provided to aid comparison; B)

water level within WELJ2 at WLS2 (15 min resolution).

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Table 1: Comparison of differing events pre- and post-WELJ installation, where WLS1

 $>0.5m^3s^{-1}$ and events precipitation >7.5mm. Storm Ciara and Dennis data highlighted.

Date	Precipitation (mm)	WLS1	WLS2	WLS3 ^{adj}	Percentage change between WLS1 and WLS3 ^{adj}
		(m³s⁻¹)	(m³s⁻¹)	(m³s⁻¹)	
Pre-installation					
09-08-19	19.8	0.71	0.31	1.44	102.6
10-08-19	14.2	0.59	0.31	0.65	10.5
11-08-19	7.6	0.5	0.3	0.59	17.2
13-08-19	14.8	0.67	0.31	0.96	43.9
14-08-19	13.6	0.57	0.3	0.71	24.7
16-08-19	24.2	0.67	0.31	1.04	53.9
Post-installation	0.0	0.50	0.05	0.50	7 6
04-09-19	8.6	0.52	0.35	0.56	7.5
11-09-19 22-09-19	8.6	0.5	0.34	0.57	13.7
22-09-19	18.8	0.53	0.37	0.61	15.1
24-09-19	12.6 15	0.6 0.59	0.39 0.39	0.78 0.62	30.6 5.4
27-09-19	21.4	0.59	0.39	0.83	34.7
28-09-19	33.6	0.01	0.4	1.71	124.8
29-09-19	24.8	0.70	0.42	0.83	16.4
06-10-19	18	0.6	0.4	0.63	4.3
09-10-19	17.6	0.63	0.4	0.68	8.3
26-10-19	20.6	0.54	0.39	0.64	18.4
01-11-19	10.4	0.58	0.36	0.56	-2.8
07-11-19	24.4	0.59	0.37	0.61	3.4
08-12-19	11.2	0.56	0.36	0.53	-6.5
10-12-19	15	0.62	0.35	0.66	7.1
12-12-19	10.4	0.53	0.35	0.47	-11.2
13-12-19	15.8	0.59	0.41	0.78	31.4
19-12-19	8	0.51	0.38	0.47	-6.5
09-01-20	11.8	0.5	0.37	0.46	-8
09-02-20	46.2	0.73	0.38	0.99	36.3
16-02-20	11.6	0.65	0.37	0.58	-11.4
Storm Ciara	46.2	0.72	1.15	0.99	27
Storm Dennis	11.2	0.65	0.74	0.58	-4.3

891 Supplementary Material

- 892 Video SM1: An Un-crewed Aerial Vehicle (UAV) fly through the Willowed Engineered Log
- Jams (WELJs) at two lads, accessible at: <u>https://www.youtube.com/watch?v=1gyfPbp4I_Y</u>
- Figure SM2: A) Catchment map of the natural flood management measures and location of
- 895 the Smithills Community at flood risk, accessible at: https://themerseyforest.sharefile.com/d-
- 896 <u>sfb38c2cbc03543be9b1f82cd0aa76a16</u>, B) An interactive edition of the map in A, accessible
- 897 at:<u>https://www.arcgis.com/apps/MapSeries/index.html?appid=5086d50ee3bc49f1bd25b039c</u>
- 898 <u>7129c1a</u> (Under: NFM Asset Map, Smithills area nr, Bolton, N.W. England)
- Figure SM3: Wildlife camera series for 28th September 07:30 30th September 12:30 event,
- 900 demonstrating inundation and attenuation with multiple channel belting, downloadable at:
- 901 https://themerseyforest.sharefile.com/d-sfa38519ebde9419f8b1cce17b85ef1c1
- 902 Video SM4: lateral inflow during high flow events (greater than approximately 0.5 m³s⁻¹
- 903 @WLS1 as in Fig 1A), downloadable at: https://themerseyforest.sharefile.com/d-
- 904 <u>s93dfa13ee83e4355accc23189e364e5e</u>